



2025 Kansas State University Master Plan Update Water Distribution System



March 12, 2013

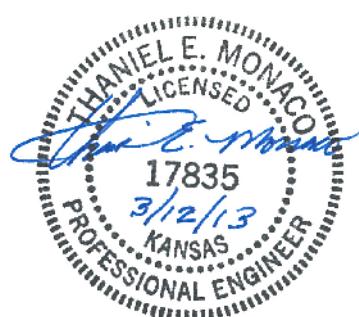


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SECTION 1

DISTRIBUTION SYSTEM

1.1. EXISTING CONDITIONS

The water distribution system that is owned and operated by Kansas State University consists of multiple types and sizes of pipe. Below is a table that summarizes the current inventory of water distribution piping which includes Ductile Iron Pipe (DIP), Cast Iron Pipe (CIP), Plastic PVC pipe, Galvanized Pipe, and Copper Pipe. The inventory was created from existing As-Built plans with the best information available to the engineer at the time of this report. A map of the water distribution system that illustrates the water main sizes (Figure 1) and a map that illustrates the water main material (Figure 2) are located at the end of this section.

Diameter (in.)	Material (Linear Feet)						
	Unknown	DIP	CIP	PVC	Galvanized	Copper	Total
3/4						217	217
1	1,659					37	1,696
1-1/4						128	128
1-1/2	28						28
1-3/4	46						46
2	911	533			247		1,691
2-1/2	31						31
3	767	330	116				1,213
4	17,910	178	2,138				20,226
6	18,502	2,725	6,979	416			28,622
8	6,625	1,565	1,414	6,979			16,583
10	4,320	2,117	6,567				13,004
16	3,814						3,814
24		803					803
Total	54,613	8,251	17,214	7,395	247	382	88,102

Table 1.1 Inventory of Existing Water Distribution System

The majority of the water distribution piping is of unknown material, however due to the age of the distribution system and the flows observed during field testing, it is expected that the majority of the unknown pipe material is cast iron.

Cast iron mains are rigid pipes that are subject to fractures and corrosion. The most common repairs to cast iron pipes include shear pipe failures and pinhole leaks. Pipe shear failure is generally caused by shifting ground and creates a complete radial fracture. Shear failures are common with nonflexible material. Corrosion and rusting of the ferrous pipe material is the cause for the other leaks within the system. Hard water in cast iron pipes also has a tendency



over time to deposit material along the inside of the pipe. This causes the pipe to become encrusted and can limit the flow capabilities. Waterlines that are made out of these types of materials can also produce brown colored water due to rust encrustation.

Currently, the Main Campus Distribution System, defined as the area of campus that is bound by Denison Ave. on the west, Manhattan Ave. on the east, Anderson Ave. on the south and Kimball Ave. on the north, including the Jardine Apartments, Edwards Hall, and the Rec Complex, is supplied by two connection points located on the corner of Kimball Ave. and Denison Ave. and north of the Center for Childhood Development building. The Kimball/Denison connection is a 24" connection that reduces down to 16" and again down to 10" before it enters the distribution system at the Power Plant. This 10" line supplies a booster pump that is located in the Power Plant.

The Power Plant booster pump has a Variable Frequency Drive (VFD) that controls the speed of the pump. The booster pump is a 75 HP Bell & Gosset/ITT Industries Centrifugal Pump rated at 1500 GPM with 150' of head. The current controls are set to turn the pump on and maintain a constant pressure in the immediate downstream piping of 80 PSI. This pump has no bypass and spins freely when downstream pressure is above 80 PSI. The connection that is located north of the Center for Childhood Development building is a 10" connection. This connection shall be referred to as the 10" Waterline Connection and the Kimball/Denison connection as the Main Supply Connection throughout this report.

At the time of this report Kansas State water distribution system is operated with the 10" Waterline Connection shut off to the main campus system. The connection is only supplying water to the Center for Childhood Development building. The remaining buildings and the entire main campus distribution system is being supplied through the Main Supply Connection. This forces all of the water that is used to flow through the booster pump with the exception of the Jardine Apartments, Edwards Hall, the Rec Complex, Pat Roberts Hall, the Vet Med Complex, Davenport Hall, Pittman Hall, and Dole Hall.

END SECTION



SECTION 2

COMPUTER MODELING

2.1. DISTRIBUTION MODELING

The K-State distribution system was modeled using a computer program called WaterCAD (Select Series 3) developed by Bentley Products. Each pipe of the distribution system was imported into the programs data base from the latest Campus Master Origin AutoCAD File provided by University personnel. The information imported included pipe length, pipe diameter, pipe material, and fire hydrants locations. The AutoCAD file and computer model was then compared to As-Built drawings that were also provided by University personnel in order to confirm waterline locations, material, and size.

Field calibration is a critical step in the development of any hydraulic analysis. Field calibration requires that the existing system be “stressed” at one or more points to evaluate how the remaining portions of the system react. The stress points within the calibration sequence are flowing fire hydrants. This is evaluated with the use of pressure gauges and pitot gauges, which will show the flowing pressure for any fire hydrant within the system.

On March 22, 2012 and March 28, 2012, BG Consultants with the assistance of University Staff conducted flow testing of numerous fire hydrants throughout the Campus. All of the calibration tests where completed with the 10" Connection open. A few tests were completed with the 10" Connection closed such as testing of the booster pump and select zone tests used to confirm system performance with this connection close. These test results are located in the Appendix.

The first portion of the fieldwork included the shutting of existing valves, where available, to provide a one-way flow within a particular pipe to assist in the evaluation of the friction factors within the pipe system. This type of testing is completed on varying sizes of water mains within the system.

The next portion of the fieldwork was the completion of a “zone-flow” analysis. This procedure requires a zone or section of the distribution system to be evaluated under normal operating conditions. The testing first requires that the static, non-flowing, pressure within the targeted section be tested. The next step in the analysis is to flow a single fire hydrant within the zone and record the system pressures at the same points within the system that the static pressures were found initially. This type of field-testing allows the computer modeler to closely resemble the actual distribution system during the development of the model.

The computer program has the ability to calibrate the model automatically using a Genetic Algorithm. The zone-flow field results are input into the model that then runs a set number of trials, typically 10,000, changing unknown variables between each run. After the trial runs, the



computer model returns set conditions for the unknown variables that will produce a model that accurately reflects that flow conditions that were experienced in the field during flow testing.

The main objective during the calibration process is to determine the coefficient of friction, or “C” factor for each type and size of pipe. The “C” factor is adjusted to reflect the reduced amount of flow a particular pipe can allow as the pipe ages or as encrustation builds. As a point of reference, new PVC pipe typically has a “C” factor around 150. After approximately 20 years under normal service conditions PVC pipe typically has a “C” factor around 140. Ductile Iron Pipe as well as Cast Iron Pipe typically has a “C” factor around 130 new and can range greatly from around 110 to 80 after 20 years. The exact amount that the “C” factor is reduced after a period of time can vary greatly depending on multiple variables including water quality and amount of water usage. A “C” factor of 60 is considered to be very low. A “C” factor that is as low as 60 is equivalent to an approximate reduction in pipe diameter of 25%. While a “C” factor of 40 is equivalent to an approximate reduction in pipe diameter of 36%. The K-State calibrated computer model produced expected “C” factors that ranged from 40 to 130. A map that illustrates the expected “C” factors for the water mains in the K-State Water Distribution System (Figure 3) is located at the end of this section.

2.2. WATER DEMAND

A very important part of the water modeling process involves accurately determining the average water demand on the distribution system that is being modeled. The demand of each user on the system must be modeled as accurately as possible as the demand affects the calibration process and the results of the predicted available fire hydrant flows. The available fire hydrant flows are the expected flow rate of each hydrant in the model that causes any point in the model to drop to a pressure of 20 PSI. The available fire hydrant flow rates are not what is expected to be experienced if the hydrant is simply opened, but what can be expected if a pump is attached to the fire hydrant and pumped at a rate that causes a point in the distribution system to drop to 20 PSI.

Water use data was collected from numerous sources that included Johnson Controls, the City of Manhattan, and K-State personnel. All of this data was evaluated in order to determine an average daily water demand for each building on the K-State Campus during a week day in a winter month and when students are in session. A table that summarizes the demands that were used in the water model is located at the end of this section. The information that was provided in order to determine the Average Demand is included in the appendix.

No water use data could be provided for roughly half of the buildings that are supplied by the main distribution system. A demand for each of these buildings was determined based on a comparison in size and usage of similar buildings that have a known demand. The total K-State **Average Daily Demand of 962,546 Gallons** was also taken into consideration when determining the demands for these buildings.



	Known Demands	Estimated Demand		Assume 10 Hrs/d w/Peak of 1
		Unit Avg.	8,448	Demand (GMP)
Brandeberry Indoor Complex	3,144			5.2
Christian Track/FB Practice Fac	6,716			11.2
Frank Myers Field	4,917			8.2
Agronomy Farms	5,431			9.1
Alumni Center				0.0
Animal Husbandry	1,877			3.1
North Farm	57,534			95.9
Putnam Hall	8,321			13.9
Smurthwaite	1,504			2.5
Stone House	685			1.1
Van Zile Hall	5,886			9.8
West Stadium	494			0.8
Bill Snyder Family Stadium	23,880			39.8
Bramlage Coliseum	4,747			7.9
Indoor Football Facility	221			0.4
Vanier Football Complex	1,099			1.8
BIVAP	2,640			4.4
Flour Mill	19,638			32.7
IGP	5,140			8.6
Edwards Hall	444			0.7
Jardine Terrace	40,248			67.1
Rec Complex	536			0.9
Ackert Hall	10,172			17.0
Ahern Field House/Gym	10,026			16.7
Anderson Hall		0.250	2,112	3.5
Beach Art Museum	1,150			1.9
Bluemont Hall		0.500	4,224	7.0
Boyd Hall	6,971			11.6
Burt Hall		0.500	4,224	7.0
Bushnell Hall		0.500	4,224	7.0
Call Hall		8.000	67,587	112.6
Calvin Hall	2,000			3.3
Cardwell Hall	11,090			18.5
Center for Child Development		0.500	4,224	7.0
Chalmers Hall	8,319			13.9
Chem/Biochem Building	11,310			18.9
Coles Hall	53,085			88.5
Danforth Chapel		0.125	1,056	1.8
Davenport Building	415			0.7



	Known Demands	Estimated Demand		Assume 10 Hrs/d w/Peak of 1
		Unit Avg.	8,448	
		Demand (GMP)		
Derby (Moore, Haymaker, Ford)	87,173			145.3
Dickens Hall		0.500	4,224	7.0
Dole Hall		0.500	4,224	7.0
Durland Hall	2,844			4.7
Dykstra Hall		0.250	2,112	3.5
East Stadium		0.125	1,056	1.8
Eisenhower Hall	1,289			2.1
English/Counseling Services		0.125	1,056	1.8
Facility Shops		0.125	1,056	1.8
Fairchild Hall	100			0.2
Feed Tech		4.000	33,793	56.3
Fiedler Hall	3,869			6.4
Galichia		0.500	4,224	7.0
Goodnow Hall	14,707			24.5
Hale	8,749			14.6
Holton Hall	750			1.3
Holtz Hall		0.125	1,056	1.8
International Student Center		0.125	1,056	1.8
Ion Lab		0.500	4,224	7.0
Justin Hall	2,458			4.1
Kedzie Hall	166			0.3
King Hall		2.000	16,897	28.2
Kramer	23,936			39.9
Leadership Studies	707			1.2
Leasure Hall	873			1.5
Marlatt Hall	27,440			45.7
McCain		0.125	1,056	1.8
Mosier Hall	244,098			406.8
Myers Building	775			1.3
Natatorium		4.000	33,793	56.3
Nichols Hall		0.125	1,056	1.8
Parking Garage	150			0.3
Pat Roberts Hall		2.000	16,897	28.2
Pittman Building	394			0.7
Power Plant (Boilers/Chill Plant)	55,457			92.4
Presidents House		0.125	1,056	1.8
Rathbone Hall	2,421			4.0
Seaton Court	17,947			29.9
Seaton Hall	42,235			70.4
Shellenberger		1.000	8,448	14.1

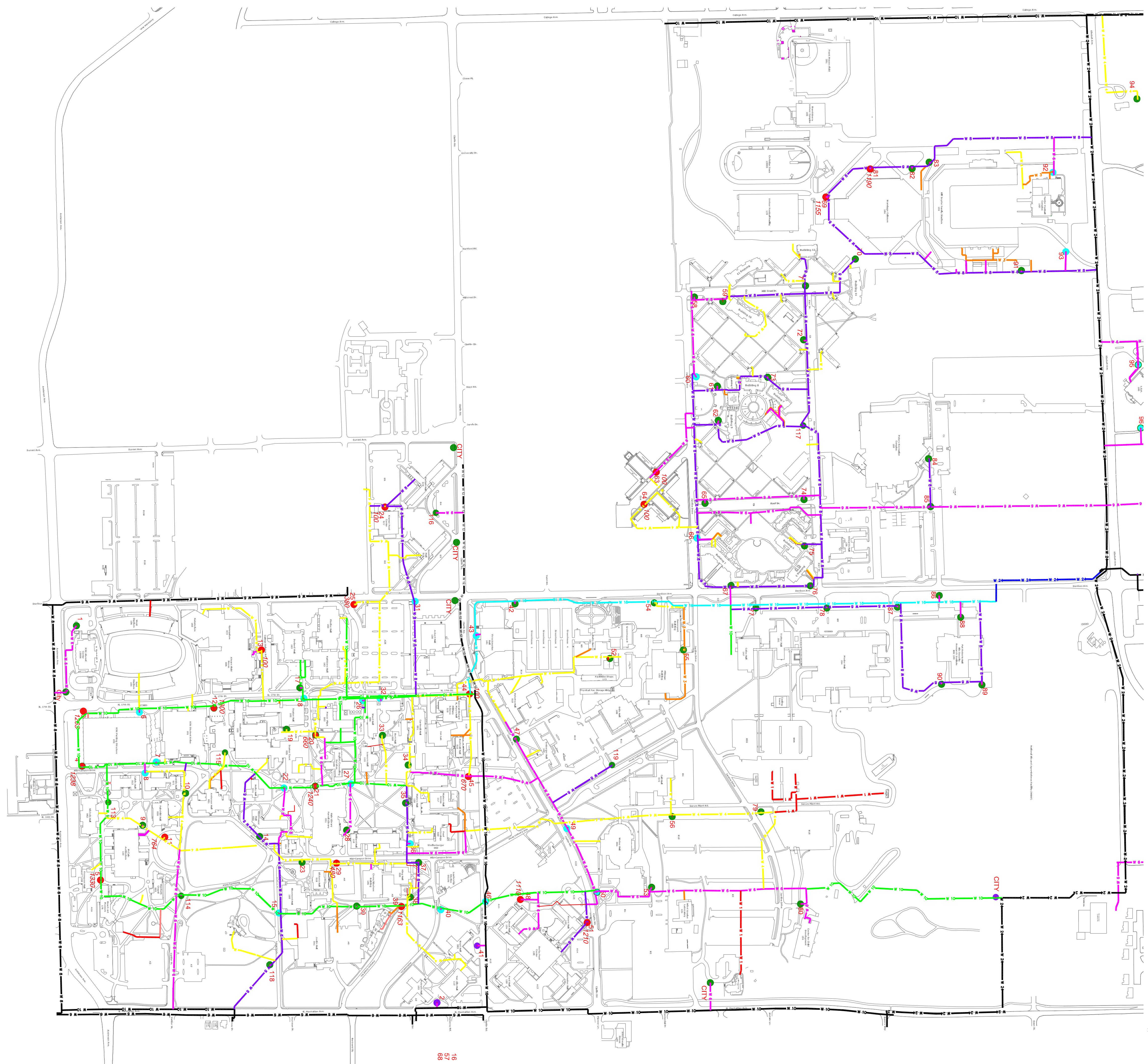


	Known Demands	Estimated Demand		Assume 10 Hrs/d w/Peak of 1 Demand (GMP)
		Unit Avg.	8,448	
Thompson Hall		0.125	1,056	1.8
Throckmorton Hall	17,663			29.4
Trotter Hall		1.000	8,448	14.1
Umberger Hall	965			1.6
Union 1(West Side?)	23,945			39.9
Union 2(East Side?)	3,625			6.0
Ward Hall	413			0.7
Waters Hall		0.500	4,224	7.0
Weber Hall		1.000	8,448	14.1
West Hall	7,399			12.3
Willard Hall	2,011			3.4
Wind Erosion Lab		0.750	6,336	10.6
Total into KSU Distribution System		962,546 GPD		1,604 GPM

Table 2.1 Summary of Average Daily Demand for each Building on K-State Campus

END SECTION





KANSAS STATE UNIVERSITY
MANHATTAN, KS

FIGURE 1A
EXISTING WATER SYSTEM - SIZING

FIGURE 1B
EXISTING WATER SYSTEM - SIZING

KANSAS STATE UNIVERSITY
MANHATTAN, KS

Engineer: BC

Drafter: JP

Project No.

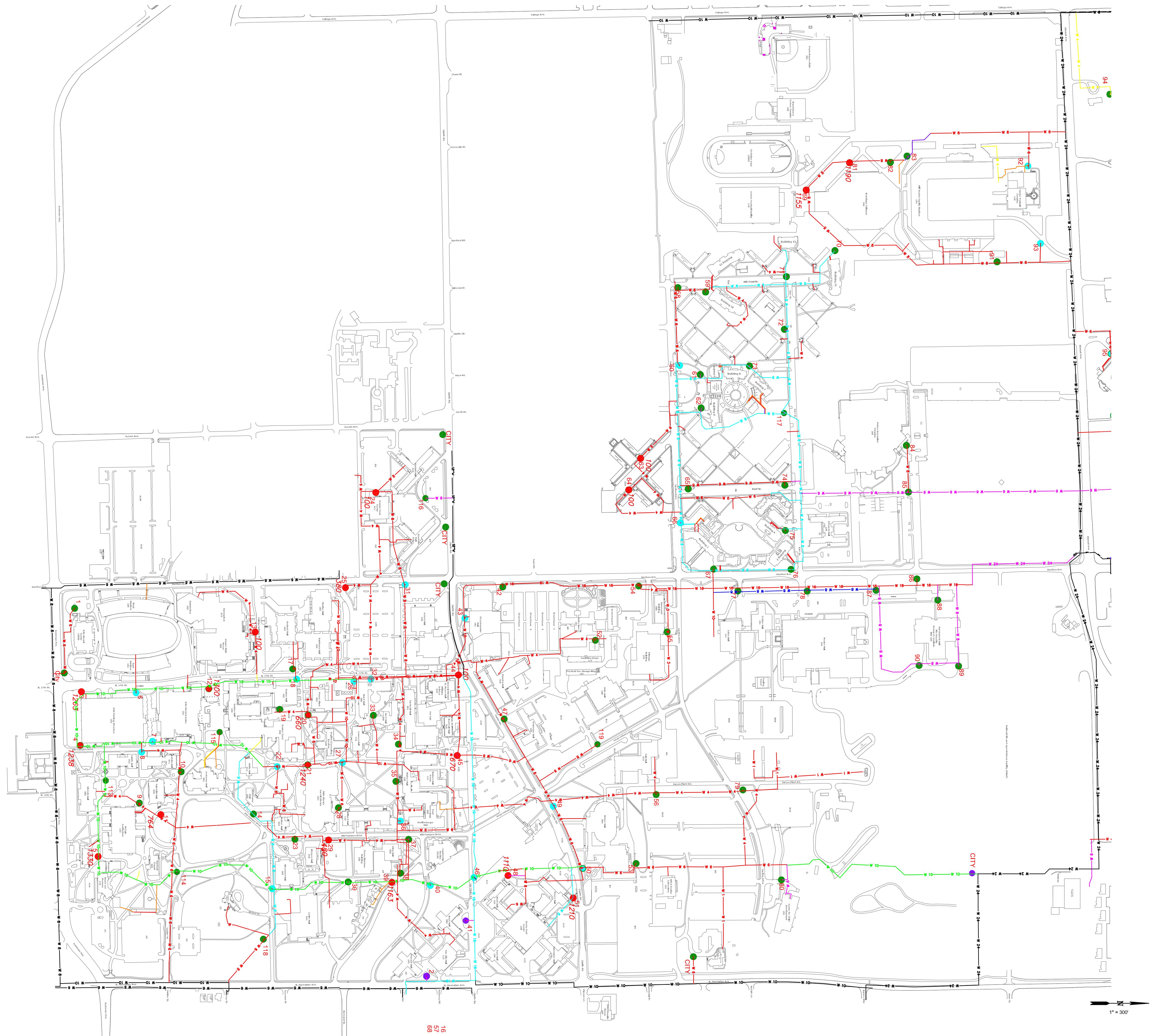
12-1078M

FIGURE
1B

LEGEND SIZING - COLOR CODING	
0.75" Water Pipe	■ - - - - -
1" Water Pipe	■ - - - - -
1.25" Water Pipe	■ - - - - -
1.5" Water Pipe	■ - - - - -
1.75" Water Pipe	■ - - - - -
2" Water Pipe	■ - - - - -
3" Water Pipe	■ - - - - -
4" Water Pipe	■ - - - - -
6" Water Pipe	■ - - - - -
8" Water Pipe	■ - - - - -
10" Water Pipe	■ - - - - -
12" Water Pipe	■ - - - - -
16" Water Pipe	■ - - - - -
24" Water Pipe	■ - - - - -
4" City Water Pipe	—
6" City Water Pipe	—
8" City Water Pipe	—
10" City Water Pipe	—
12" City Water Pipe	—
16" City Water Pipe	—
24" City Water Pipe	—
Fire Hydrant - No Testing	●
Fire Hydrant - Flow Test	●
Fire Hydrant - Static/Residual Test	●
Fire Hydrant - Number	1
Fire Hydrant - Flow	100

1" = 300'





KANSAS STATE UNIVERSITY
MANHATTAN, KS

EXISTING WATER SYSTEM - MATERIAL

BG CONSULTANTS, INC.
ENGINEERS - ARCHITECTS - SURVEYORS
MANHATTAN, KANSAS
HUTCHINSON, KANSAS
LAWRENCE, KANSAS
EMPIORA, KANSAS

B G
CONSULTANTS INC.
HUTCHINSON, KANSAS
LAWRENCE, KANSAS
EMPIORA, KANSAS

Engineer: BC
Drafter: JP
Project No.
12-1078M

FIGURE
2A

FIGURE 2B
EXISTING WATER SYSTEM - MATERIAL

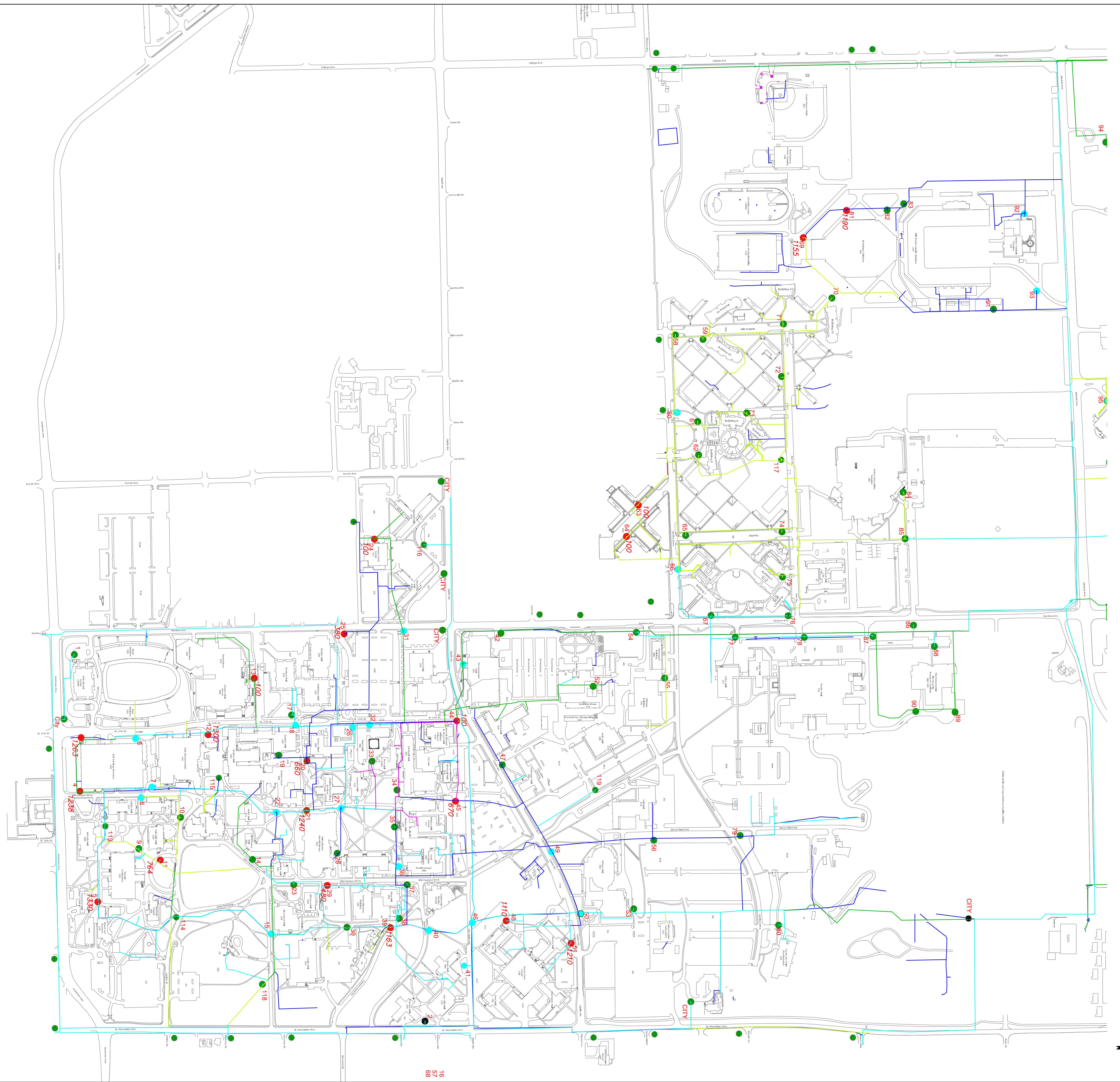
FIGURE 2B

KANSAS STATE UNIVERSITY
MANHATTAN, KS

LEGEND MATERIAL - COLOR CODING			
City	PVC	CIP	Copper
			DIP
			Unknown
			Galvanized
●	●	●	●
FIRE HYDRANT - NO TESTING	FIRE HYDRANT - FLOW TEST	FIRE HYDRANT - STATIC/RESIDUAL TEST	FIRE HYDRANT - NUMBER
1	100		
100			

Engineer: BC	Drafter: JP
Project No. 12-1078M	

FIGURE
2B



KANSAS STATE UNIVERSITY
MANHATTAN, KS

FIGURE 3A
EXISTING WATER SYSTEM - "C" FACTORS

NO. REVISIONS DATE INITIALS

BG CONSULTANTS, INC.
ENGINEERS - ARCHITECTS - SURVEYORS
MANHATTAN, KANSAS
HUTCHINSON, KANSAS
LAWRENCE, KANSAS
EMPIORA, KANSAS

B G
CONSULTANTS INC.

Engineer: BC
Drafter: JP
Project No.
12-1078M

FIGURE
3A

KANSAS STATE UNIVERSITY
MANHATTAN, KS

FIGURE 3B
EXISTING WATER SYSTEM - "C" FACTORS

LEGEND "C" FACTORS	
—	<= 20.0
—	<= 40.0
—	<= 60.0
—	<= 80.0
—	<= 100.0
—	<= 130.0
—	Other
●	Fire Hydrant - No Testing
●	Fire Hydrant - Flow Test
●	Fire Hydrant - Static/Residual Test
1	Fire Hydrant - Number
100	Fire Hydrant - Flow

1° = 300'

Engineer: BC
Drafter: JP
Project No.
12-1078M
FIGURE
3B

SECTION 3

DISTRIBUTION SYSTEM ANALYSIS

3.1. SYSTEM PRESSURES

In analyzing the distribution system for any community, one of the main concerns is the pressures provided at all points throughout the system. The Kansas Department of Health and Environment (KDHE) recommends a minimum of **20 psi** in waterlines under all flow conditions. In general, we do not recommend that service pressure fluctuate more than **10 psi**. We also recommend a service range of **60 psi to 80 psi** for public water supply systems. We analyzed the existing system and found that static pressures throughout the system are typically well above the recommended value between 80 and 100 psi. During normal usage we found that the water distribution system works within KDHE requirements.

3.2. FIRE FLOW PROTECTION

Current KDHE regulations require a minimum of 6" diameter waterlines provide fire protection. Approximately 28.7% of the waterlines within the existing distribution system are smaller than 6" and 20 out of the 114 existing fire hydrants are supplied by a waterline that is smaller than 6". The National Fire Protection Agency (NFPA) standard color coding for available fire hydrant flow rates at 20 psi is listed below.

<u>Classification</u>	<u>Flow at 20 psi.</u>	<u>Color Code</u>
Class C	Less than 500 gpm	Red
Class B	500 to 999 gpm	Orange
Class A	1000 to 1499 gpm	Green
Class AA	1500 and above	Blue

During testing 7 out of the 27 fire hydrants tested could not achieve a flow rate greater than 500 gpm. After creating a distribution model using WaterCAD, it was found that 17 out of the 114 total fire hydrants could not achieve 500 gpm while maintaining 20 psi within the system during normal demand usage with the 10" Connection open. A detailed fire code analysis would be required to determine the recommend flows for each hydrant due to the various size and usage of each building across the main campus. However, it typically is recommend that all fire hydrants be able to flow a minimum of 500 gpm and that fire hydrants near large facilities, such as schools, dormitories, etc. be able to flow a minimum of 1500 gpm.

Table A.1, located in the appendix, lists the available fire flow at each hydrant with the 10" Connection both opened and closed. As shown in table A.1, most of the fire hydrants cannot meet the recommended flow of 1500 gpm.



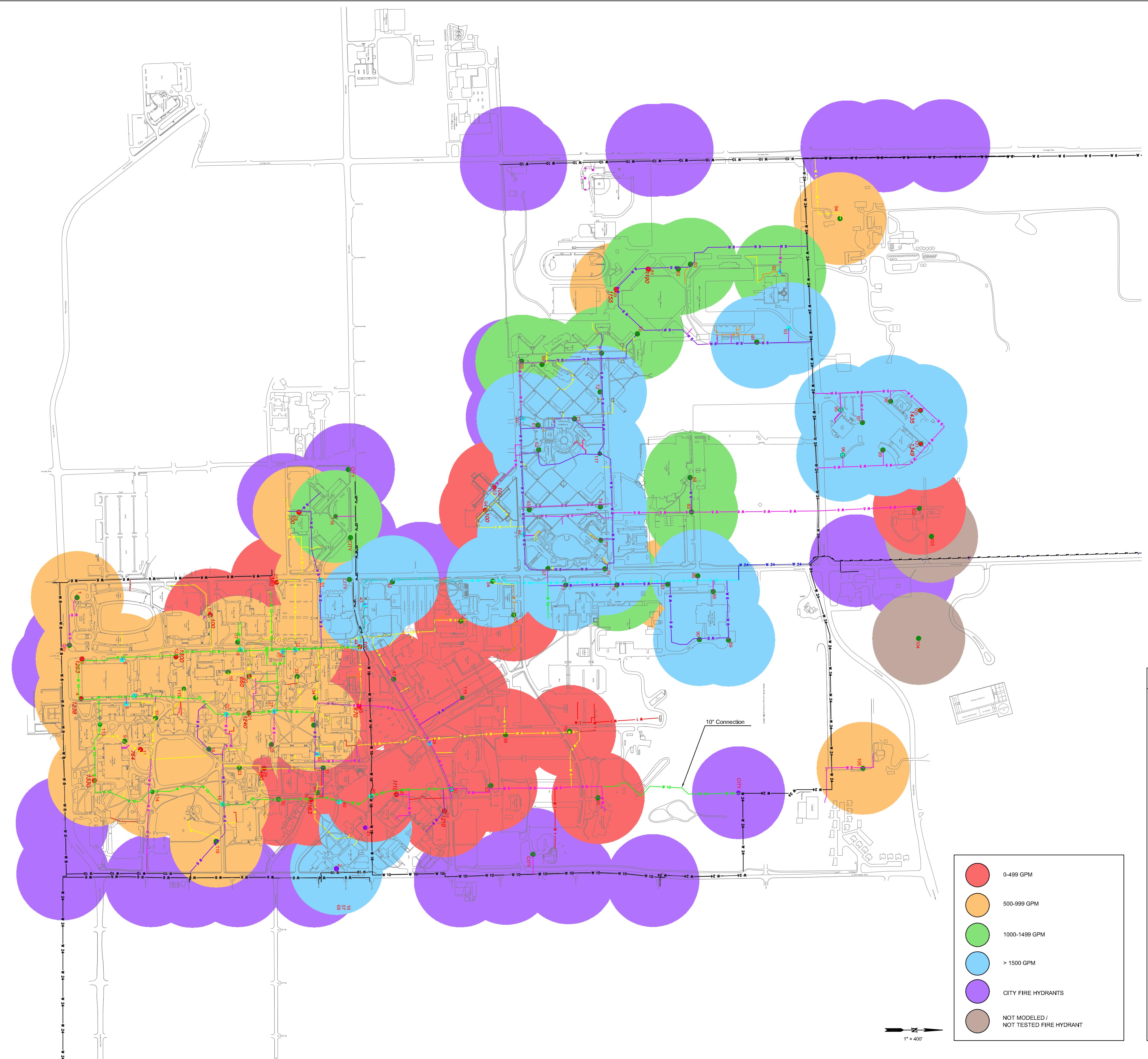
3.3. FIRE HYDRANT SPACING

Fire hydrant spacing is largely set by individual cities and is normally at the discretion of the Fire Chief. This spacing is largely dependent upon the amount of hose each fire truck has available. The objective is that any location within the protection area can be reached by one or more fire hydrants of ample flow. The minimum standard for most cities require fire hydrants to be placed no further than 800 feet apart. This would allow any location to be accessed by 400 feet of fire hose.

The Kansas Department of Health and Environment *Policies, General Considerations, and Design Requirements for Public Water Supply Systems in Kansas* states that fire hydrants should be placed at each street intersection and at intermediate points between intersections. Generally, hydrant spacing may range from 350 feet to 600 feet. This is the same standard as the Ten-States Standard (1997) and as recommended by the State Insurance Service Office. Two maps were created to illustrate the recommended 400 feet radius around each existing fire hydrant within the distribution system. These maps illustrate both the coverage and the NFPA Classification of each fire hydrant located within the distribution system and are located at the end of this section. Figure 4, illustrates the system coverage and available flows with the 10" Connection closed while Figure 5 illustrates the system coverage and available flows with the 10" Connection open.

END SECTION





KANSAS STATE UNIVERSITY
MANHATTAN, KS

AVAILABLE FIRE HYDRANT - 10" CONNECTION - CLOSED

FIGURE 4A

BG CONSULTANTS, INC.	
ENGINEERS - ARCHITECTS - SURVEYORS	
MANHATTAN, KANSAS	HUTCHINSON, KANSAS
LAWRENCE, KANSAS	EMPIORA, KANSAS
INITIALS	
DATE	
REVISIONS	
NO.	

FIGURE
4A

Engineer: BC
Drafter: JP
Project No.
12-1078M

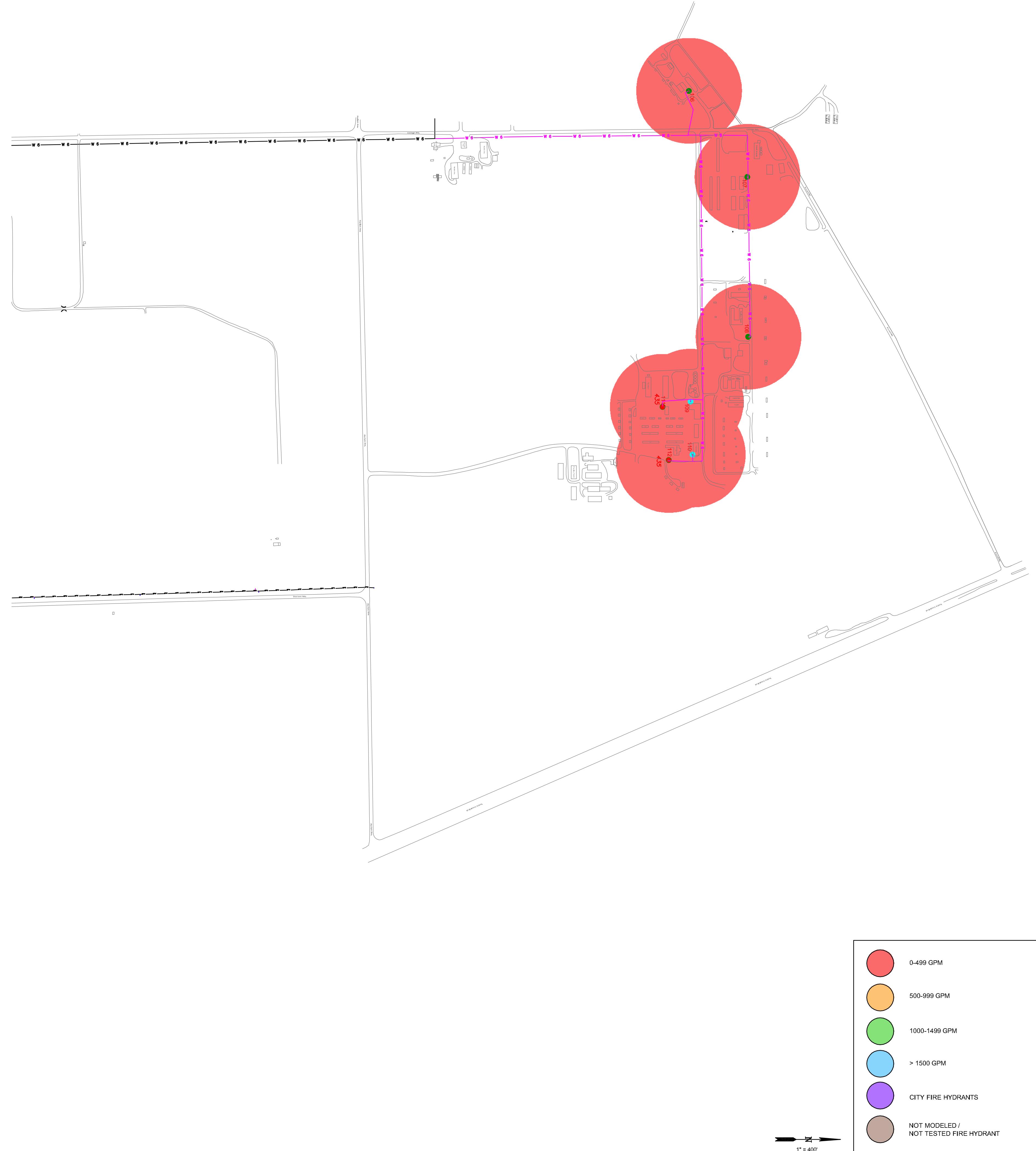
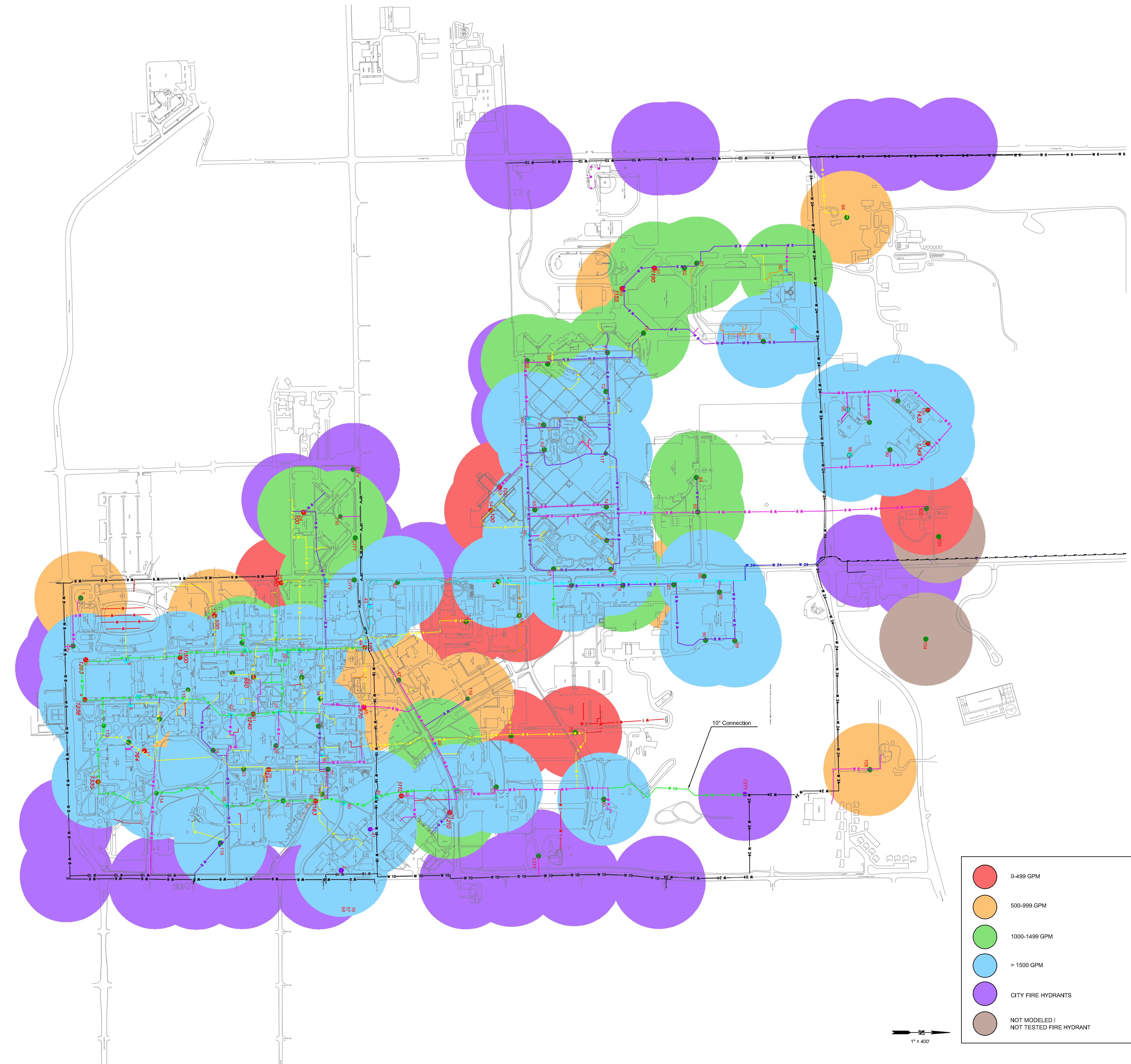


FIGURE 5A
AVAILABLE FIRE HYDRANT - 10" CONNECTION - OPEN





SECTION 4

DISTRIBUTION SYSTEM DEFICIENCIES AND RECOMMENDATIONS

4.1. AREAS OF DEFICIENCIES

The existing Kansas State University Potable Water Distribution has a few deficiencies, most of the deficiencies result from areas of campus that are not covered by fire hydrants that can produce the recommended flow rate of 1500 gpm at 20 psi. As illustrated in Figure 6, located at the end of this section, there are five main areas of concern.

It should be noted that there are couple of additional deficiencies that are not noted on Figure 6. They include the existing booster pump, fire hydrant flow rates around the Football Stadium, fire hydrant flow rates around the Recreation Complex, fire hydrant flow rates in the area to the northwest of the intersection of Kimball and Denison Avenue, and a few various fire hydrants that are located in isolated areas that are on dead end waterlines. These areas are considered secondary deficiencies and are discussed below.

4.2. SECONDARY DEFICIENCIES

4.2.1. EXISTING BOOSTER PUMP

As mentioned before, the Main Supply Connection enters the K-State Power Plant that supplies a 1500 gpm booster pump. The booster pump does not have a bypass and when the service pressure downstream of the pump is above 80 psi, the pump spins freely as water flows through it. This causes a drop in pressure between 3 and 17 psi, depending on the flow rate through the pump. This can reduce pressure system wide and could potentially force the booster pump to turn on more often than is necessary.

The booster pump also has the potential to limit the available fire hydrants flow rates. The maximum designed flow rate for the pump is 1500 gpm. Therefore when a fire hydrant is opened, that hydrant can only flow at a maximum of 1500 gpm. The flow rate would be expected to be less than 1500 gpm due to friction losses as the water travels through the system.

The booster pump was originally installed to maintain K-State distribution system pressure during times when the City of Manhattan could not supply the required pressure. Since that time, the City of Manhattan has completed multiple improvement project that have decreased the amount of pressure fluctuations that the KSU Campus experiences. In addition, the second 10" Waterline Connection that was installed also helps to reduce pressure fluctuations.



It is recommended that a booster pump bypass be installed to allow for the free flow of water around the booster pump during times when the pump is not needed. However, as stated before this is considered a secondary deficiency and should be kept in mind for any future improvement plans. It is estimated that a booster bypass installation would cost approximately \$50,000 or less.

4.2.2. FOOTBALL STADIUM

As illustrated in Figure 5, the available fire hydrant flow at the time this report was completed, show fire hydrants that are expected to produce less than 1500 gpm at 20 psi. This is considered to be a secondary deficiency. The West Stadium Expansion project and the Basketball Training Facility were both under construction at the time that the computer water model was created. There is a potential that some gate valves were closed or that some waterlines were not in place during field testing. It is expected that upon completion of the construction projects, the fire hydrants in this area will produce the recommended fire hydrant flow rate. Therefore, at this time these fire hydrants are considered a secondary deficiency. It is recommended that these fire hydrants be tested upon the completion of construction and the results are included into the computer model in order to confirm the available flow rates.

4.2.3. RECREATION COMPLEX

The Rec Complex Expansion project was also under construction during the time this report was completed. The expansion project included the replacement of a number of fire hydrants in this area. It was unfeasible to include these fire hydrants in this evaluation and is therefore considered to be a secondary deficiency. It is recommended that these fire hydrants also be tested upon the completion of construction and the results are included into the computer model as well.

4.2.4. NW OF THE INTERSECTION OF KIMBALL AND DENISON

Fire hydrant 102 is located at the end of a dead end 6" waterline. This fire hydrant is located inside of an animal pin and is almost buried. This area has experienced recent improvements including the installation of additional fire hydrants that are served off of a City of Manhattan 12" waterline. These new fire hydrants were not included in the field test. The new fire hydrants are expected to produce available fire hydrant flows greater than 1500 gpm at 20 psi, therefore the hydrants in this area of campus are considered to be secondary deficiencies.

4.2.5. ISOLATED FIRE HYDRANTS ON DEAD END LINES

There are a few areas located throughout the campus that cannot produce the recommended flow rate. In most cases, improvements to the City of Manhattan Distribution System would be required in order to improve the flow rate of these hydrants. These fire hydrants are also



usually covered by other fire hydrants that are expected to produce the recommended flow rate. Therefore, these hydrants are considered to be secondary deficiencies.

4.3. MAJOR DEFICIENCIES

There are 5 areas of the K-State Campus that are considered to be major deficiencies. They include area #1, the Engineering complex/Kramer Complex, Area #2, the South Jardine Apartments, Area #3, Umberger/Call Hall/Dole Hall/Mosier Hall, Area #4, the Derby Complex, and Area #5, the North Farm. Each of these areas is described below and is illustrated in Figure 6. The recommended improvements that are aimed at addressing the noted deficiencies are also illustrated in Figure 6. A detailed EOPC of each recommendation is included below.

4.3.1. AREA #1 ENGINEERING COMPLEX/KRAMER COMPLEX

This area includes the following buildings: Natatorium, Ahearn Gymnasium, Fiedler Hall, Kramer Dinning Hall, and Goodnow and Marlatt Residence Halls. The following fire hydrants are not expected to produce the recommended flow rate of 1500 gpm, hydrant number 13, 17, 25, 24, 116, and 31. This recommendation will replace a 4" waterline that supplies the Natatorium and Ahearn Field House and Gym. It will also replace a 4" waterline that runs from the power plant to the Kramer Dinning Center. This recommendation will include the installation of a new waterline the runs along Denison Ave. from the Natatorium to Fiedler Hall.

Engineers Opinion of Probable Project Cost					
Area #1					
No.	Description	Qty.	Unit	Unit Price	Total Price
1	Mobilization and Incidental		Lump Sum	\$ 50,000.00	\$ 50,000.00
2	10" PVC Waterline (In Place)	1700	LF	\$ 40.00	\$ 68,000.00
3	10" PVC Waterline (Bore)	1100	LF	\$ 70.00	\$ 77,000.00
4	Tracer Wire	2800	LF	\$ 0.50	\$ 1,400.00
5	Misc. Fittings		Lump Sum	\$33,000.00	\$ 33,000.00
6	Fire Hydrant Setting	5	EA	\$ 4,000.00	\$ 20,000.00
7	Connect to Existing Waterline	4	EA	\$ 2,000.00	\$ 8,000.00
8	Flowable Fill	165	CY	\$ 90.00	\$ 14,850.00
9	Remove and Replace Surfacing	330	SY	\$ 100.00	\$ 33,000.00
		10%	Contingency		\$ 30,525.00
		Total Construction Cost			\$ 335,775.00
		Design Engineering			\$ 83,943.75
		Total Opinion of Cost			\$ 419,718.75

Table 4.1 – Area #1 EOPC



4.3.2. AREA #2 SOUTH JARDINE APARTMENT

This area includes the following buildings: Jardine Terrace H, I, N, and M. The following fire hydrants are not expected to produce the recommended flow rate of 1500 gpm, hydrant number 63 and 64. This recommendation will replace the 6" and 4" loop that services these four apartment buildings.

Engineers Opinion of Probable Project Cost					
Area #2					
No.	Description	Qty.	Unit	Unit Price	Total Price
1	Mobilization and Incidental		Lump Sum	\$22,000.00	\$ 22,000.00
2	10" PVC Waterline (In Place)	700	LF	\$ 40.00	\$ 28,000.00
3	10" PVC Waterline (Bore)	600	LF	\$ 70.00	\$ 42,000.00
4	Tracer Wire	1300	LF	\$ 0.50	\$ 650.00
5	Misc. Fittings		Lump Sum	\$14,000.00	\$ 14,000.00
6	Fire Hydrant Setting	3	EA	\$ 4,000.00	\$ 12,000.00
7	Connect to Existing Waterline	2	EA	\$ 2,000.00	\$ 4,000.00
8	Flowable Fill	30	CY	\$ 90.00	\$ 2,700.00
9	Remove and Replace Surfacing	60	SY	\$ 100.00	\$ 6,000.00
		10%	Contingency		\$ 13,135.00
			Total Construction Cost		\$144,485.00
			Design Engineering		\$ 36,121.25
			Total Opinion of Cost		\$180,606.25

Table 4.2 – Area #2 EOPC



4.3.3. AREA #3 UMBERGER/CALL HALL/DOLE HALL/MOSIER HALL

This area includes the following buildings: Military Science Building, Bushnell, Dykstra, Umberger, International Student Center, Dole, Pittman, Various Facility Shops, Call, and Mosier Halls. The following fire hydrants are not expected to produce the recommended flow rate of 1500 gpm, hydrant number 44, 45, 47, 49, 52, 55, 56, 75, and 119. This recommendation will replace a 6" waterline that runs along Claflin Road from Throckmorton to Weber Hall and then north to the Center for Childhood Development Center. This recommendation includes the installation of a waterline the runs along Jardine Road from Denison Ave. to the Center for Childhood Development. It also includes the installation of a waterline that follows part of Mid Campus drive from Claflin Road north to Jardine Road, this waterline will continue behind the Vet Complex and connect to the Pat Roberts Hall Waterline Loop. From here the waterline will continue east along the south property line of the proposed NBAF facility and connect to the existing 10" waterline at a point north of the Center for Childhood development.

Engineers Opinion of Probable Project Cost					
Area #3					
No.	Description	Qty.	Unit	Unit Price	Total Price
1	Mobilization and Incidentals		Lump Sum	\$ 110,000.00	\$ 110,000.00
2	12" PVC Waterline (In Place)	3900	LF	\$ 45.00	\$ 175,500.00
3	12" PVC Waterline (Bore)	400	LF	\$ 90.00	\$ 36,000.00
4	10" PVC Waterline (In Place)	3900	LF	\$ 40.00	\$ 156,000.00
5	10" PVC Waterline (Bore)	400	LF	\$ 70.00	\$ 28,000.00
6	Tracer Wire	4300	LF	\$ 0.50	\$ 2,150.00
7	Misc. Fittings		Lump Sum	\$ 72,000.00	\$ 72,000.00
8	Fire Hydrant Setting	17	EA	\$ 4,000.00	\$ 68,000.00
9	Connect to Existing Waterline	9	EA	\$ 2,000.00	\$ 18,000.00
10	Flowable Fill	80	CY	\$ 90.00	\$ 7,200.00
11	Remove and Replace Surfacing	165	SY	\$ 100.00	\$ 16,500.00
		10%	Contingency		\$ 68,935.00
		Total Construction Cost			\$ 758,285.00
		Design Engineering			\$ 189,571.25
		Total Opinion of Cost			\$ 947,856.25

Table 4.3 – Area #3 EOPC



4.3.4. AREA #4 DERBY COMPLEX

This area includes the following buildings: Derby Dinning Center and the Haymaker Residence Hall. The following fire hydrants are not expected to produce the recommended flow rate of 1500 gpm, hydrant number 51. This location also has an area that is not within 400' of any fire hydrants. This recommendation will include the installation of a waterline loop that will wrap around the Derby Dining Center and the four adjacent Residence Halls.

Engineers Opinion of Probable Project Cost					
Area #4					
No.	Description	Qty.	Unit	Unit Price	Total Price
1	Mobilization and Incidentals		Lump Sum	\$33,000.00	\$ 33,000.00
2	10" PVC Waterline (In Place)	1200	LF	\$ 40.00	\$ 48,000.00
3	10" PVC Waterline (Bore)	800	LF	\$ 70.00	\$ 56,000.00
4	Tracer Wire	2000	LF	\$ 0.50	\$ 1,000.00
5	Misc. Fittings		Lump Sum	\$22,000.00	\$ 22,000.00
6	Fire Hydrant Setting	6	EA	\$ 4,000.00	\$ 24,000.00
7	Connect to Existing Waterline	2	EA	\$ 2,000.00	\$ 4,000.00
8	Flowable Fill	50	CY	\$ 90.00	\$ 4,500.00
9	Remove and Replace Surfacing	100	SY	\$ 100.00	\$ 10,000.00
		10%	Contingency		\$ 20,250.00
		Total Construction Cost			\$ 222,750.00
		Design Engineering			\$ 55,687.50
		Total Opinion of Cost			\$ 278,437.50

Table 4.4 – Area #4 EOPC



4.3.5. AREA #5 NORTH FARM

This area includes the all of the buildings and services in located at the North Farm. The following fire hydrants are not expected to produce the recommended flow rate of 1500 gpm, hydrant number 106, 107, 108, 109, 110, 111, and 112. This recommendation will include the installation of a waterline from the intersection of Marlatt and Denison Avenues north to connect with the existing water system that serves the North Farm.

Engineers Opinion of Probable Project Cost					
Area #5					
No.	Description	Qty.	Unit	Unit Price	Total Price
1	Mobilization and Incidental		Lump Sum	\$45,000.00	\$ 45,000.00
2	12" PVC Waterline (In Place)	3240	LF	\$ 45.00	\$ 145,800.00
3	Tracer Wire	3240	LF	\$ 0.50	\$ 1,620.00
4	Misc. Fittings		Lump Sum	\$22,000.00	\$ 22,000.00
5	Fire Hydrant Setting	4	EA	\$ 4,000.00	\$ 16,000.00
6	Connect to Existing Waterline	2	EA	\$ 2,000.00	\$ 4,000.00
7	Remove and Replace Surfacing	80	SY	\$ 15.00	\$ 1,200.00
		10%	Contingency		\$ 23,562.00
					\$ 259,182.00
					Design Engineering
					\$ 64,795.50
					Total Opinion of Cost
					\$ 323,977.50

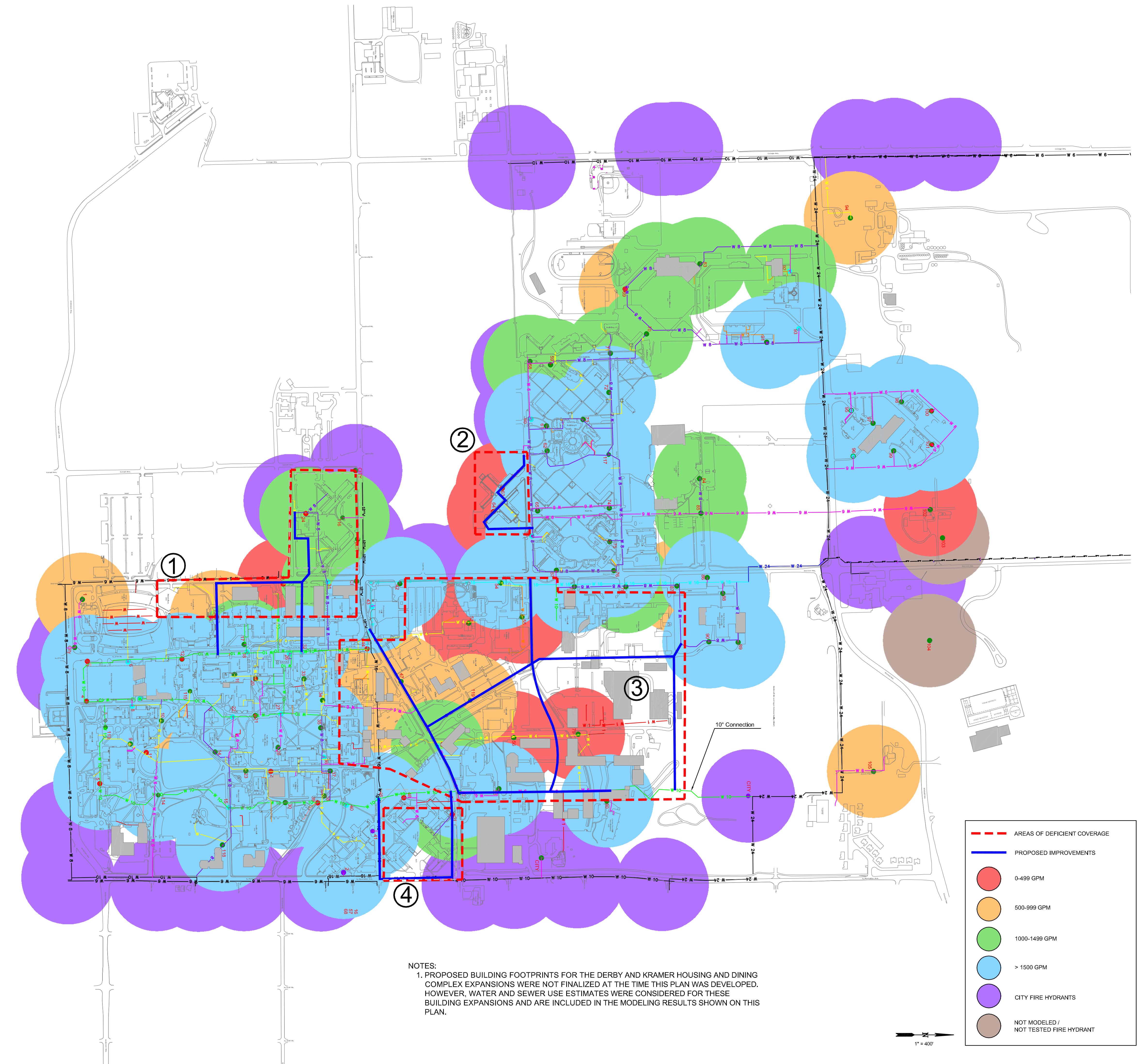
Table 4.5 – Area #5 EOPC

4.4. FUTURE EXPANSION

The current distribution system along with the previously recommended improvements, totaling **\$2,150,596.25**, will provide adequate domestic water supply and fire protection based on the projected future building locations as detailed in the 2025 Campus Master Plan.

END SECTION

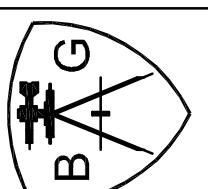




KANSAS STATE UNIVERSITY
MANHATTAN, KS

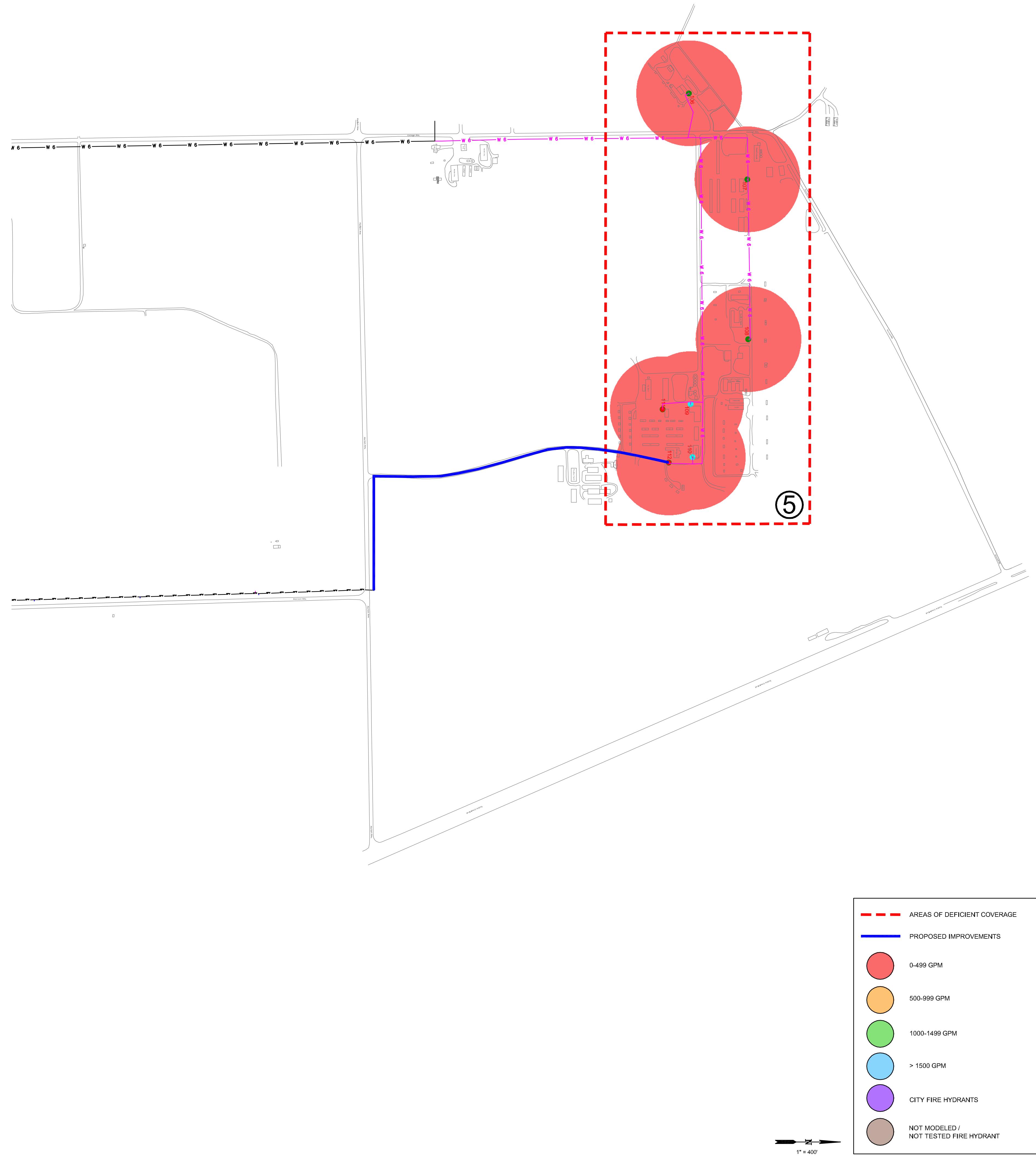
FIGURE 6A
AREAS OF DEFICIENT FIRE HYDRANT COVERAGE

BG CONSULTANTS, INC.
ENGINEERS - ARCHITECTS - SURVEYORS
MANHATTAN, KANSAS HUTCHINSON, KANSAS LAWRENCE, KANSAS EMPORIA, KANSAS



NO.	REVISIONS	DATE	INITIALS

Engineer: BC	Drafter: JP
Project No. 12-1078M	
FIGURE 6A	



KANSAS STATE UNIVERSITY
MANHATTAN, KS

FIGURE 6B
AREAS OF DEFICIENT FIRE HYDRANT COVERAGE

Engineer: BC
Drafter: JP
Project No.
12-1078M

FIGURE
6B

APPENDIX

- Fire Hydrant Field Flow Testing Results
- Booster Pump Station Testing Results
- Provided Water Consumption Data
- Table A.1 – Available Fire Hydrant Flow Rates



Kansas State University
Flow Testing Project No. 12-1078M
3/22/12 to 3/23/12

Zone Flow			Pressure (psi)		Pressure	Field Flow	Calculated		
Run No.	Hydrant No.	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	3/22/2012	
1B	4					70.0	1238	Flow	Diffuser
1C	6		109	70	39			Test	
1D	7		100	65	35			Test	
1A	3					74.0	1263	Flow	Diffuser
1C	6		109	74	35			Test	
1D	7		100	70	30			Test	
1A	3							Flow	Diffuser
1B	4							Flow	Diffuser
1C	6		109	38	71			Test	
1D	7		100	33	67			Test	
Zone Flow			Pressure (psi)		Pressure	Field Flow	Calculated		
Run No.	Hydrant No.	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	3/28/2012 9:26	
2A	5					63.0	1330	Flow	
2C	8		97	61	36			Test	
2D	15		105	69	36			Test	
2B	11					24.0	764	Flow	Diffuser
2C	8		97	71	26			Test	
2D	15		105	79	26			Test	
2A	5					45.0	1125	Flow	
2B	9					12.0	537	Flow	Diffuser
2C	8		97	42	55			Test	
2D	15		105	48	57			Test	
Zone Flow			Pressure (psi)		Pressure	Field Flow	Calculated		
Run No.	Hydrant No.	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	3/28/2012 9:52	
3A	12					60.0	1300	Flow	
3C	18		93	62	31			Test	
3D	22		95	65	30			Test	
3B	13					0.0	100	Flow	Diffuser
3C	18		93	90	3			Test	
3D	22		95	90	5			Test	
3A	12					53.0	1220	Flow	
3B	13					0.0	100	Flow	Diffuser
3C	18		93	56	37			Test	
3D	22		95	58	37			Test	
Zone Flow			Pressure (psi)		Pressure	Field Flow	Calculated		
Run No.	Hydrant No.	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	3/28/2012 10:15	
4A	20					18.0	660	Flow	Diffuser
4C	26		92	72	20			Test	
4D	27		90	72	18			Test	
4B	21					55.0	1240	Flow	
4C	26		92	62	30			Test	
4D	27		90	64	26			Test	
4A	20					10.0	493	Flow	Diffuser
4B	21					37.0	1025	Flow	
4C	26		92	42	50			Test	
4D	27		90	45	45			Test	
Zone Flow			Pressure (psi)		Pressure	Field Flow	Calculated		
Run No.	Hydrant No.	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	2/28/2012 8:55	
5A	24					0.0	100	Flow	Diffuser
5C	31		93	80	13			Test	
5D	32		96	92	4			Test	
5B	25					5.0	380	Flow	
5C	31		93	86	7			Test	
5D	32		96	91	5			Test	
5A	24					0.0	100	Flow	Diffuser
5B	25					3.0	290	Flow	
5C	31		93	72	21			Test	
5D	32		96	88	8			Test	
Zone Flow			Pressure (psi)		Pressure	Field Flow	Calculated		
Run No.	Hydrant No.	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	2/28/2012 10:37	
6A	29					8.0	480	Flow	
6C	26		98	85	13			Test	
6D	40		104	95	9			Test	
6B	39					60.0	1163	Flow	Diffuser
6C	36		98	58	40			Test	
6D	40		104	64	40			Test	
6A	29					5.0	380	Flow	
6B	39					50.0	1077	Flow	Diffuser
6C	36		98	49	49			Test	
6D	40		104	55	49			Test	

**Kansas State University
Flow Testing Project No. 12-1078M
3/22/12 to 3/23/12**

One Way Flow			Pressure (psi)		Pressure	Field Flow	Calculated	Comments
Run Size	FH #	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	
8"	87					40.0	974	Flow - Diffuser
	78		100	48	52			Test
	77		102	82	20			Test

6"	48					22.0	790	Flow - No Diffuser
	50		104	33	71			Test
	53		106	38	68			Test

							Flow
							Test
							Test

							Flow
							Test
							Test

							Flow
							Test
							Test

							FLOW
							Test
							Test

							Flow
							Test
							Test

Kansas State University
Flow Testing Project No. 12-1078M
6/18/2012

Zone Flow		Pressure (psi)		Pressure	Field Flow	Calculated		
Run No.	Hydrant No.	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	
13A	5					60.0	1300	Flow
13C	4		101	68	33			Test
13D	7		98	58	40			Test
13E	114		112	70	42			Test
13A	5					36.0	1010	Flow
13B	113					27.0	875	Flow
13C	4		101	40	61			Test
13D	7		98	32	66			Test
13E	114		112	44	68			Test
13B	113					52.0	1210	Flow
13C	4		101	64	37			Test
13D	7		98	58	40			Test
13E	114		112	69	43			Test

Zone Flow		Pressure (psi)		Pressure	Field Flow	Calculated		
Run No.	Hydrant No.	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	
14A	21					59.0	1290	Flow
14C	20		106	70	36			Test
14D	26		104	66	38			Test
14E	27		105	70	35			Test
14A	18					30.0	920	Flow
14B	21					33.0	965	Flow
14C	20		106	48	58			Test
14D	26		104	38	66			Test
14E	27		105	41	64			Test
14B	18					52.0	1210	Flow
14C	20		106	64	42			Test
14D	26		104	64	40			Test
14E	27		105	68	37			Test

Zone Flow		Pressure (psi)		Pressure	Field Flow	Calculated		
Run No.	Hydrant No.	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	
15A	29					10.0	530	Flow
15C	36		108	98	10			Test
15D	40		114	115	-1			Test
15A	29		100			5.0	380	Flow
15B	39					55.0	1240	Flow
15C	36		108	60	48			Test
15D	40		114	60	54			Test
15B	39					68.0	1385	Flow
15C	36		108	70	38			Test
15D	40		114	74	40			Test

**Kansas State University
Flow Testing Project No. 12-1078M
3/22/12 to 3/23/12**

One Way Flow			Pressure (psi)		Pressure	Field Flow	Calculated	Comments
Run Size	FH #	Location	Static	Residual	Loss (psi)	(psi)	Flow (gpm)	
10	6					55.0	1265	Flow
	12		115	64	51			Test
	18		100	58	42			Test

							Flow
							Test
							Test

							Flow
							Test
							Test

							Flow
							Test
							Test

							Flow
							Test
							Test

							Flow
							Test
							Test

							Flow
							Test
							Test

Kansas State University
Flow Testing Project No. 12-1078M
6/1/2012

Time	Up Stream Pressure	Actual Meter Reading (Units in 6 Sec.)	Booster Meter Reading (CCF/Min)	Booster Meter Reading (GPM)	Predicted FH Flow (GPM)	Down Stream Pressure	Fire Hydrant (Actual)		Fire Hydrant (Target)		Difference in Predicted Flow and Actual Flow (GPM)	Headloss Across Pump (PSI)	Headloss Across Pump (FT)
							Pressure	Flow Rate	Pressure	Flow Rate			
Before	110	6	0.6	449	0	93.5	0	0	0	0	0	16.5	38.115
8:30	105	11	1.1	823	374	93.2	6	410	6	410	36	11.8	27.258
	103	12	1.2	898	449	92.8	9	500	9	500	51	10.2	23.562
	102	14	1.4	1,047	598	92.1	13	605	13	605	7	9.9	22.869
	101	16	1.6	1,197	748	91.9	18	710	18	710	-38	9.1	21.021
	100	17	1.7	1,272	823	91.7	23	805	23	805	-18	8.3	19.173
	99	18	1.8	1,346	898	91.1	29	905	29	905	7	7.9	18.249
	98	19	1.9	1,421	972	90.5	36	1010	36	1010	38	7.5	17.325
	96	20	2	1,496	1,047	89.7	43	1100	43	1100	53	6.3	14.553
9:40	92	22	2.2	1,646	1,197	89.2	50	1190	51	1200	-7	2.8	6.468
							X	X	60	1300			
							X	X	70	1405			
							X	X	80	1500			

City of Manhattan

Units: CCF

Read Date	Putnam-Wtr
1/27/2009	183
2/25/2009	315
3/27/2009	280
4/21/2009	301
5/27/2009	216
6/26/2009	158
7/24/2009	176
8/27/2009	256
9/28/2009	524
10/28/2009	436
11/24/2009	248
12/28/2009	215
1/27/2010	115
2/26/2010	273
3/29/2010	180
4/28/2010	248
5/27/2010	188
6/25/2010	102
7/28/2010	133
8/27/2010	372
9/27/2010	407
10/27/2010	395
11/23/2010	262
12/28/2010	215
1/27/2011	130
2/28/2011	385
3/29/2011	184
4/25/2011	276
5/31/2011	255
6/24/2011	112
7/25/2011	234
8/24/2011	276
9/26/2011	568
10/25/2011	487
11/22/2011	347
12/23/2011	258
1/25/2012	131
2/24/2012	389
3/27/2012	320

Read Date	Van Zile-Wtr
1/27/2009	293
2/25/2009	286
3/27/2009	389
4/21/2009	370
5/27/2009	285
6/26/2009	165
7/24/2009	99
8/21/2009	227
9/27/2009	403
10/26/2009	428
11/23/2009	352
12/22/2009	298
1/27/2010	240
2/25/2010	392
3/23/2010	361
4/22/2010	436
5/23/2010	297
6/23/2010	68
7/25/2010	102
8/25/2010	198
9/26/2010	456
10/26/2010	416
11/22/2010	323
12/22/2010	230
1/23/2011	83
2/23/2011	359
3/27/2011	255
4/26/2011	309
5/26/2011	197
6/26/2011	68
7/26/2011	164
8/24/2011	148
9/25/2011	401
10/25/2011	364
11/22/2011	279
12/22/2011	227
1/24/2012	142
2/22/2012	325
3/26/2012	280

Read Date	Smurthwaite-Wtr
1/27/2009	29
2/27/2009	73
3/26/2009	55
4/21/2009	65
5/28/2009	38
6/29/2009	5
7/27/2009	3
8/21/2009	17
9/24/2009	67
10/28/2009	46
11/24/2009	43
12/24/2009	29
1/27/2010	25
2/26/2010	53
3/29/2010	40
4/28/2010	54
5/27/2010	32
6/25/2010	1
7/29/2010	3
8/26/2010	29
9/27/2010	78
10/26/2010	71
11/23/2010	63
12/28/2010	48
1/27/2011	30
2/28/2011	72
3/29/2011	51
4/27/2011	60
5/31/2011	42
6/28/2011	6
7/28/2011	47
8/30/2011	57
9/27/2011	75
10/31/2011	82
11/30/2011	46
12/28/2011	35
1/30/2012	42
2/28/2012	66
3/29/2012	46

Account Name	Account #
KSU Putnam Hall	11500-11501
KSU Van Zyle Hall	11504-11505
KSU Van Zyle & Putnam Irr	11872-11873
KSU Smurthwaite	11514-11515

Meters have been combined in tracking under "Van Zile-Water"

HOUSING WATER METERS, MAY 7-11, 2012

READINGS

	X	PAGE	19-Apr	7-May	8-May	9-May	10-May	11-May
REC COMPLEX	W0210	0.1337	21	702341	702341	702342	702342	702342
REC COMPLEX, HANDBA	W0220	1	21	164	167	168	168	168
EDWARDS	W0025,HIGH	0.1337	21	3542	3557	3558	3559	3560
EDWARDS	W0025,LOW	0.01337	21	135730	136366	136411	136457	136497
DAVENPORT	W0415	1	29	5444	5452	5452	5453	5454
JARDINE	W0450	1	30	41533	41533	41533	41533	41533
JARDINE	W0455	1	30	9197	327	389	452	515
PITTMAN, IRRIGATION	W0460	0.1337	30	102150	102158	102158	102158	102158
PITTMAN	W0430	1	30	26467	26474	26474	26475	26475
MARLATT	W0425	1.337	31	10983	11521	11571	11623	11678
KRAMER	W0420	1	32	32779	33210	33244	33279	33309
GOODNOW	W0410	1	33	84757	85324	85382	85440	85514
DERBY	W0405	0.1337	34	26382	48006	49040	50099	51155
WEST	W0445	1	34	296480	296739	296752	296768	296781
BOYD	W0400	1	36	10355	10616	10628	10650	10680
VAN ZILE	W0440	0.1	37	6580	6805	6820	6834	6847
PUTNAM	W0435	0.1	37	41136	n/a	n/a	43924	n/a
SMURTHWAITE	NOT THRU FACILITIES							

HOUSING WATER METERS, MAY 7-11, 2012

USAGE in CCF

	X	PAGE	19-Apr	7-May	8-May	9-May	10-May	11-May
REC COMPLEX	W0210	0.1337	21	0	0	0	0	0
REC COMPLEX, HANDBA	W0220	1	21	3	1	0	0	0
EDWARDS	W0025,HIGH	0.1337	21	2	0	0	0	0
EDWARDS	W0025,LOW	0.01337	21	9	1	1	1	1
DAVENPORT	W0415	1	29	8	0	1	1	0
JARDINE	W0450	1	30	0	0	0	0	0
JARDINE	W0455	1	30	1130	62	63	63	64
PITTMAN, IRRIGATION	W0460	0.1337	30	1	0	0	0	0
PITTMAN	W0430	1	30	7	0	1	0	1
MARLATT	W0425	1.337	31	719	67	70	74	20
KRAMER	W0420	1	32	431	34	35	30	32
GOODNOW	W0410	1	33	567	58	58	74	25
DERBY	W0405	0.1337	34	2891	138	142	141	119
WEST	W0445	1	34	259	13	16	13	12
BOYD	W0400	1	36	261	12	22	10	20
VAN ZILE	W0440	0.1	37	23	2	1	1	1
PUTNAM	W0435	0.1	37	n/a	n/a	279	n/a	n/a
SMURTHWAITE	NOT THRU FACILITIES				20 Days use			

Kansas State University
2025 University Master Plan Update
Water Distribution System
 BG Project # 10-1078M July 5, 2012

Table A.1 - Available Fire Hydrant Flow Rates

0-499	500-999	1000-1499	1500+	
Fire Hydrant Number	Flow Rate (GPM)		Additional Flow w/10" Connection Open	
	Existing Conditions			
	10" Open	10" Closed		
1	699	699	0	
2	3488	3488	0	
3	1721	536	1185	
4	1729	535	1194	
5	1740	528	1212	
6	1703	538	1165	
7	1739	534	1205	
8	1515	534	981	
9	962	529	433	
10	791	531	260	
11	912	529	383	
12	1537	542	995	
13	538	395	143	
14	1736	527	1209	
15	1756	519	1237	
17	1490	549	941	
18	1570	549	1021	
19	341	274	67	
20	720	523	197	
21	1755	549	1206	
22	1644	541	1103	
23	778	517	261	
24	1080	548	532	
25	291	232	59	
26	1587	556	1031	
27	1762	553	1209	
28	880	553	327	
29	856	511	345	
30	1769	497	1272	
31	1272	548	724	
32	1584	553	1031	
33	234	188	46	
34	607	428	179	
35	1648	520	1128	
36	1633	501	1132	
37	1774	493	1281	
38	1154	487	667	
39	1778	487	1291	
40	1791	472	1319	
41	2811	2811	0	
42	5000	3422	1578	

Kansas State University
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Table A.1 - Available Fire Hydrant Flow Rates

0-499	500-999	1000-1499	1500+	
Fire Hydrant Number	Flow Rate (GPM)		Additional Flow w/10" Connection Open	
	Existing Conditions			
	10" Open	10" Closed		
43	5000	3217	1783	
44	512	384	128	
45	920	483	437	
46	1782	452	1330	
47	841	474	367	
48	1219	435	784	
49	1180	446	734	
50	1679	420	1259	
51	1429	421	1008	
52	331	260	71	
53	1510	342	1168	
54	2155	2119	36	
55	336	334	2	
56	266	173	93	
58	1426	1406	20	
59	1435	1415	20	
60	1562	1538	24	
61	1587	1563	24	
62	1652	1625	27	
63	177	176	1	
64	177	176	1	
65	1726	1698	28	
66	1771	1741	30	
67	2052	2014	38	
69	991	989	2	
70	1329	1310	19	
71	1474	1452	22	
72	1530	1507	23	
73	1616	1591	25	
74	1723	1694	29	
75	1764	1734	30	
76	1802	1771	31	
77	2025	1996	29	
78	1017	1008	9	
79	318	181	137	
80	1982	249	1733	
81	1045	1044	1	
82	1144	1142	2	
83	1238	1236	2	
84	1049	1038	11	
85	1071	1059	12	

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Table A.1 - Available Fire Hydrant Flow Rates

0-499	500-999	1000-1499	1500+	
Fire Hydrant Number	Flow Rate (GPM)		Additional Flow w/10" Connection Open	
	Existing Conditions			
	10" Open	10" Closed		
86	3992	3953	39	
87	759	752	7	
88	2612	2595	17	
89	4128	4091	37	
90	4775	4727	48	
91	1965	1961	4	
92	1475	1473	2	
93	1779	1776	3	
94	579	578	1	
95	2176	2172	4	
96	2441	2436	5	
97	1705	1703	2	
98	1959	1956	3	
99	1944	1941	3	
100	2014	2011	3	
101	1995	1992	3	
102	452	448	4	
105	768	768	0	
106	409	408	1	
107	396	396	0	
108	396	396	0	
109	394	394	0	
110	382	381	1	
111	394	394	0	
112	383	382	1	
113	1703	532	1171	
114	1730	525	1205	
115	575	425	150	
116	1404	1404	0	
117	1646	1619	27	
118	1756	518	1238	
119	972	456	516	